



TOWN OF AMHERST

AMHERST INDUSTRIAL PARK EAST DEVELOPMENT PHASE 1A

CONTRACT NO. RFT-26-05

ADDENDUM NO. 1

October 21, 2026

(To be added to and made part of the Tender Documents)

The following changes or modifications shall be made to the Tender Documents:

TO THE SPECIFICATIONS

00 21 13 – INFORMATION TO TENDERERS

Page 4, Add new clause 24 as follows

24. A Geotechnical Investigation Report has been carried out at the Site. The Geotechnical Investigation Report, dated July 3, 2026, prepared by CBCL Limited, is attached as Appendix A.

APPENDIX A - GEOTECHNICAL INVESTIGATION REPORT

Add new Appendix A as attached.

TO THE CONTRACT

CCDC 41 – INSURANCE REQUIREMENTS

Page 1, Delete Clauses 3, 4, 5 and 6 and renumber subsequent.

CBCL Limited
October 21, 2026

APPENDIX A

GEOTECHNICAL INVESTIGATION REPORT



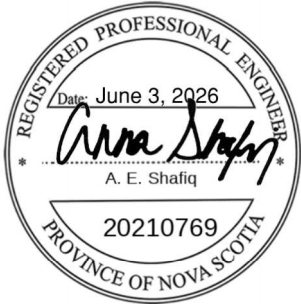
GEOTECHNICAL INVESTIGATION

Amherst Industrial Park Expansion
Amherst, NS

Town of Amherst

263404.00 • June 3, 2026



0	Draft	RC/AES	3-Jun-2026	AGM
Rev.	Issue	Reviewed By:	Date	Issued By:
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June 3, 2026

Aaron Bourgeois
Town of Amherst
98 East Victoria Street
Amherst, NS
B4H 1X6

Email: abourgeois@amherst.ca

Dear Mr. Bourgeois,

RE: Geotechnical Investigation – Amherst Industrial Park Expansion - Amherst, NS

Please find attached CBCL Limited's Geotechnical Investigation Report in support of the proposed road for the Amherst Industrial Park Expansion project. This report presents our findings from the field investigation program and our geotechnical recommendations for general site works and pavement design.

Yours very truly,

CBCL Limited



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Project No.: 263404.00

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- A Test pit Location Plan
- B Symbols and Terms Used on Test pit Records
- C Test pit Records
- D Guidelines for Winter Construction

1 Introduction

At the request of the Town of Amherst, CBCL Limited (CBCL) has conducted a geotechnical investigation in support of the proposed roadway at the Amherst Industrial Park in Amherst, NS.

The objective of the geotechnical investigation was to evaluate subsurface soil and groundwater conditions at sixteen (16) locations along the proposed roadway and provide geotechnical recommendations for roadway construction.

This report presents our findings during the geotechnical investigation at the site.

This report has been prepared for the sole benefit of the Town of Amherst and was prepared specifically for the subject site. The findings and recommendations contained in this report should not be extrapolated beyond the area investigated.

2 Site Description and Geology

The subject site is made up of seven (7) properties (PIDs 25444845, 25037193, 25037185, 25070608, 25047200, 25037169 and 25395419), located on Industrial Park Drive in Amherst, NS. The roadways investigated total approximately 1.7 km in length including a realignment of the main access road to the industrial park, a T-intersection and a road providing access to the new commercial lots.

The soil and rock conditions encountered at the test pit locations represent a limited sampling of each site and change of conditions is highly possible. The data obtained at each test pit location can only be extrapolated to an undefined, limited area surrounding the test pit locations. Notwithstanding, it is intended that each test pit will give an indication of the soil and rock conditions to be encountered at the investigated site.

Surficial geologic mapping indicates that the area generally consists of Silty Till Plain and is described as silty, compact material derived from both local and distant sources¹. Bedrock in the area is mapped as part of the Balfron Formation of the Pictou Group and is described as fluvial sandstone, conglomerate, floodplain mudstone, mudstone and rare lacustrine limestone².

¹ Stea, R.R., Conley, H., Brown, Y. (1992). Surficial Geology of the Province of Nova Scotia – Map 92-3.

² Keppie, J.D. (2000). Geological Map of the Province of Nova Scotia – Map ME 2000-1

3 Fieldwork Procedure

The field program consisted of sixteen (16) test pits and were completed on April 13th and 14th, 2026. The test pit locations are shown in Drawing SK1 - Appendix A and summarized in Table 1.

The test pits were advanced using a track-mounted excavator supplied and operated by Beale & Inch Construction Ltd. of Sackville, NB. Test pits were loosely backfilled with excavated materials to reestablish pre-excavation grades.

Test pit locations were selected based on conceptual layout of the proposed roads.

The soil samples were classified by visual examination according to the Standard Practice for Description and Identification of Soils (Visual-Manual Procedures) (ASTM D2488).

Following completion of the fieldwork, soil samples were taken to CBCL's laboratory in Moncton, NB, where additional testing and evaluation were carried out on representative samples. It should be noted that the samples will be stored in the laboratory for a period of three (3) months following the date of the report, after which they will be discarded unless instructed otherwise.

Test pit locations and elevations were surveyed in the field using a Leica GNSS GPS unit.

Table 1 Test pit Location Summary

Test pit ID	Northing ¹ (m)	Easting ¹ (m)	Surface Elevation ² (m)	Completion Depth (m)
TP-01	5073408.54	405966.48	21.8	2.0
TP-02	5073530.21	406056.39	21.9	1.8
TP-03	5073652.44	406171.27	25.7	2.0
TP-04	5073580.09	406337.93	24.4	2.2
TP-05	5073593.46	406498.40	16.7	2.6
TP-06	5073498.06	406317.64	23.6	2.7
TP-07	5073407.03	406243.30	22.2	2.5
TP-08	5073321.77	406181.51	16.7	2.3
TP-09	5073245.38	406121.92	20.2	0.5
TP-10	5073167.51	406005.05	18.5	2.7
TP-11	5073156.90	405862.22	19.6	2.5
TP-12	5073205.27	405747.71	21.6	2.2
TP-13	5073280.78	405654.46	22.0	2.8
TP-14	5073358.18	405560.64	22.9	2.3
TP-15	5073367.91	406499.64	21.6	2.2
TP-16	5073448.87	406483.72	21.3	3.0

Notes: ¹ Coordinate System: NAD 83, UTM 20N

² Geodetic Datum: CGVD28

4 Summarized Subsurface Conditions

The subsurface conditions encountered in the Test Pits are summarized in Table 2, described in the following sections, and described in detail in the appended Test Pit Records (Appendix C). For an explanation of the descriptions used on the Test Pits Records, reference should be made to the appended Symbols and Terms used on Test Pits Records (Appendix B).

The subsurface soil conditions encountered in the test pits mainly consist of the following profile:

- ▶ ROOTMAT/TOPSOIL
- ▶ FILL or Silty/Clayey SAND
- ▶ TILL

Table 2 Subsurface Conditions Summary

Test Pit ID	Thickness of:				Depth to		Depth of Test Pit (m)
	ROOTMAT / TOPSOIL (mm)	FILL (m)	Silty Clayey SAND (m)	TILL (m)	GW Seepage ² (m)	Inferred Bedrock (m)	
TP-01	600	-	-	>1.4	-	-	2.0
TP-02	600	-	-	>1.2	-	-	1.8
TP-03	600	-	-	>1.4	-	-	2.0
TP-04	600	-	-	>1.6	-	-	2.2
TP-05	400	-	-	>2.2	1.0	-	2.6
TP-06	500	-	-	>2.2	-	-	2.7
TP-07	600	-	0.4	>1.5	-	-	2.5
TP-08	600	0.4	-	>1.3	-	-	2.3
TP-09	500	-	-	-	-	0.5	0.5
TP-10	800	0.3	-	>1.6	-	-	2.7
TP-11	300	-	1.9	>0.3	-	-	2.5
TP-12	300	-	0.5	>1.4	-	-	2.2
TP-13	300	-	-	>2.5	-	-	2.8
TP-14	300	-	-	>2.0	-	-	2.3
TP-15	300	-	-	>1.9	-	-	2.2

Test Pit ID	Thickness of:				Depth to		Depth of Test Pit (m)
	ROOTMAT / TOPSOIL (mm)	FILL (m)	Silty Clayey SAND (m)	TILL (m)	GW Seepage ² (m)	Inferred Bedrock (m)	
TP-16	-	0.9	-	2.1	-	3.0	3.0

Note: 1 "-" Not observed/Not measured.
2 Groundwater levels were inferred from visible water seepage.
3 A thickness shown with a greater than sign (>) means the test pit was terminated within this stratum and therefore the layer thickness was not measured.

4.1 ROOTMAT / TOPSOIL

A layer of ROOTMAT and TOPSOIL was encountered at ground surface at all test pit locations except for test pits TP-16. The thickness of this layer ranged from approximately 300 to 800 mm. This layer can generally be described as a loose brown topsoil and rootmat. Ground covering ranged from low lying grasses, small trees and older growth trees.

A grain size analysis test was conducted on one (1) representative sample obtained from this layer. The gradation analysis results are included in and summarized in Table 4.

Table 3 Gradation Analysis Results Summary – TOPSOIL and ROOTMAT

Sample ID	Depth (m)	Moisture (%)	Gravel (%)	Sand (%)	Fines (%)
TP-9	0.2	22	7	62	31

4.2 FILL

A layer of FILL with an approximate thickness ranging from 0.3 to 0.9 m was encountered at surface in test pit TP-16 and below the ROOTMAT and TOPSOIL in test pits TP-08 and TP-10. The FILL is mainly described as brown to greyish brown, silty/clayey sand to sandy silt/clay.

4.3 Silty/Clayey SAND

A layer of silty/clayey SAND with an approximate thickness ranging from 0.4 to 1.9 m was encountered below the ROOTMAT and TOPSOIL in test pits TP-07, TP-11 and TP-12. The SAND is mainly described as loose to compact, reddish brown, silty/clayey sand.

4.4 TILL

Native TILL was encountered below the ROOTMAT and TOPSOIL or FILL in all test pits with the exception of TP-09. This layer was encountered at depths ranging from approximately 0.3 to 2.2 m. Till can vary significantly as a native, heterogeneous mixture of clay, silt, sand, and gravel and cobbles. The native TILL at this site can generally be described as reddish

brown, silty/clayey SAND with gravel and cobbles. The relative density of this layer was generally inferred to be compact to dense.

A grain size analysis test was conducted on fifteen (15) representative samples obtained from this layer. The gradation analysis results are included in and summarized in Table 4.

Table 4 Gradation Analysis Results Summary – Native Soil

Sample ID	Depth (m)	Moisture (%)	Gravel (%)	Sand (%)	Fines (%)
TP-1	1.3	14	25	50	25
TP-2	1.2	11	25	44	31
TP-3	1.3	14	9	54	37
TP-4	1.4	14	3	21	76
TP-5	1.5	15	27	57	16
TP-6	1.6	13	23	43	34
TP-7	1.7	13	8	47	46
TP-8	1.6	15	4	53	43
TP-10	1.9	15	7	50	43
TP-11	1.4	15	17	56	27
TP-12	1.3	13	37	44	19
TP-13	1.5	11	11	46	43
TP-14	1.3	12	11	44	45
TP-15	1.2	14	20	54	26
TP-16	1.9	17	5	56	39

Please refer to the appended test pit logs for a more detailed description of the soils encountered during the geotechnical investigation.

4.5 Groundwater

Groundwater seepage was observed in test pit TP-05 at a depth of 1.0 m below the ground surface.

Groundwater levels should be expected to fluctuate seasonally, with precipitation, site development, and/or with construction activity.

4.6 Bedrock

Bedrock was inferred in test pits TP-09 and TP-16 at depths of 0.9 m and 3.0 m, respectively.

5 Discussion and Recommendations

5.1 Earthworks

Earthwork for this project will include excavation of the existing TOPSOIL and ROOTMAT and fill soils to the depth required to construct the recommended cross-section outlined below.

5.1.1 Surface Water Control and Erosion Control

Prior to excavations, surface water drainage controls should be provided on the up-gradient side of the site to minimize run-off onto exposed soils, especially in areas where engineered fills will be placed to construct/reconstruct the roads. Suitable erosion and sedimentation control measures should be employed. These may include silt fences, check dams in ditches, granular working pads and mechanical pumps.

5.1.2 Excavation

Safe excavation is the responsibility of the earthwork contractor, and as a minimum must follow provincial guidelines. Based on the subsurface conditions, temporary excavated side slopes in dry soil conditions should be stable at one horizontal to one vertical (1H:1V) for the anticipated excavation depths throughout the site. Saturated slopes should be reviewed by geotechnical personnel during construction.

Excavation slopes should be checked regularly for signs of instability and flattened as required. Temporary slopes should be protected from surface erosion by means such as (but not limited to): swales or ditches around the perimeter of the excavation; and/or by using plastic sheeting placed over the slope.

Stockpiled material, excavated soil, and wheel loads should be kept away from the excavation crest by a distance equal to the depth of the excavation. It is recommended to have a geotechnical engineer review excavations for safety and stability on a regular basis during construction.

Material that is planned for re-use should be placed directly in the intended areas or compacted in stockpiles for later use. Materials unsuitable for use as structural fill should be used in landscape areas or transported off-site to an approved location.

5.1.3 Dewatering of Excavations

The contractor undertaking the earthworks must be prepared to dewater excavations. Discharge from the dewatering activities must be carried out in strict accordance with environmental regulations.

5.1.4 Fill Placement and Compaction

Based on the results of laboratory testing completed on the existing soil, it was determined that the materials have a high fines content (>10%) that varies significantly across the project site. Therefore, the existing soils are not recommended for reuse as structural fill for the road reconstruction. If the contractor believes a material is suitable for reuse, a Geotechnical Engineer should review and provide guidance.

Structural fill required throughout the project site should consist of the following:

- ▶ Approved, imported, quarried NSDPW Type 1 or Type 2 gravels

The lift thickness used during the placement of fill must be compatible with the compaction equipment and the material type to ensure the specified density throughout. For preliminary consideration, the lift thickness should not exceed 200 mm in its loose state. The maximum particle size should be no larger than $\frac{2}{3}$ of the lift thickness.

Fill should be placed and compacted at or slightly below optimum moisture content (ASTM D698). Moisture conditioning may be needed to achieve the optimum moisture content. The in-place density of the fill should be tested for acceptance during construction by qualified geotechnical personnel. Fill should not be placed on frozen ground, slough, or in standing water.

Fill should be compacted to the following percentage of maximum Standard Proctor dry density:

▶ Road Base and Subbase.....	100%
▶ Within 300 mm of roadway subbase	98%
▶ More than 300 mm below the roadway subbase	95%
▶ Embankment Backfill.....	95%
▶ Landscaped Areas.....	93%

5.1.5 Roadway Subgrade

During earthworks, the contractor must take precautions to avoid disturbance of the existing site soils or reinstate the material to the required condition. The condition of the subgrade should be reviewed by a geotechnical engineer prior to placement of aggregates.

Exposed subgrade surfaces should be proof rolled with a 10-tonne static roller or a loaded tandem truck in the presence of a geotechnical engineer, prior to placement of the proposed

pavement structure materials. Weak zones encountered at the subgrade level should be removed and reinstated with structural fill as directed by the geotechnical engineer.

Adequate drainage (subsurface and/or ditching) should be in place to remove water from within the pavement structure. The drainage system should be such that the pavement structure remains adequately drained. Lack of drainage will cause premature deterioration of the pavement structure.

5.1.6 Roadway Structure

It is understood that paved roadway will be constructed/reconstructed for the proposed development.

With the subgrade prepared as outlined in Section 5.1 the pavement structure in Table 5 is recommended.

Table 5 Pavement Structure Thicknesses

Material		Thickness (mm)
Asphalt	Top Course (D mix)	40
	Base Course (B Mix)	110
Aggregate Base (NSDPW Type 1 gravel)		150
Aggregate Subbase (NSDPW Type 2 gravel)		450

Please be advised that CBCL did not undertake a traffic analysis to estimate the ESALs for this project. If precise design ESALs is desired, then a traffic study should be conducted.

All aggregate and asphalt concrete materials should meet the NSDPW Standard Specifications for Highway Construction and Maintenance. The aggregates should be compacted as outlined in Section 5.1.4. Asphalt concrete should be compacted to 92.5% of Maximum Theoretical Relative Density.

Review of the placement and compaction operations for the pavement structure (i.e. base gravels and asphalt pavement) should be carried out by qualified geotechnical personnel.

5.2 Observation and Testing

Additional geotechnical input is recommended during the final design stage to ensure that the project fully and accurately considered/interpreted all the information and recommendations from the geotechnical investigation.

Observation and testing are recommended during site grading and backfilling operations.

5.3 Winter Construction

Additional effort will be required to conduct earthwork during winter conditions. Winter construction guidelines are attached in Appendix D.

Undertaking earthworks in the winter is not ideal and despite best efforts and good intentions, the quality of earthworks is often compromised.

6 Closure

This report has been prepared for the sole benefit of the Client. All information, documentation, or other material contained in, attached to, or forming part of this report reflects CBCL's opinion and best judgment based on the information available to us at the time of preparation. Any use or reliance on this report by the Client in circumstances where there has been a change in site conditions or for any purpose not expressly intended by or delineated in this report shall be the sole responsibility of the Client and CBCL accepts no liability for such use or reliance. Any use or reliance on this report by any third party, without CBCL's prior express written consent, shall be the sole responsibility of that third party. CBCL accepts no liability whatsoever for such use or reliance.

The information and conclusions contained in this report are generally consistent with professional standards for engineering and scientific professionals providing similar services at the same time, in similar locations, and under similar circumstances.

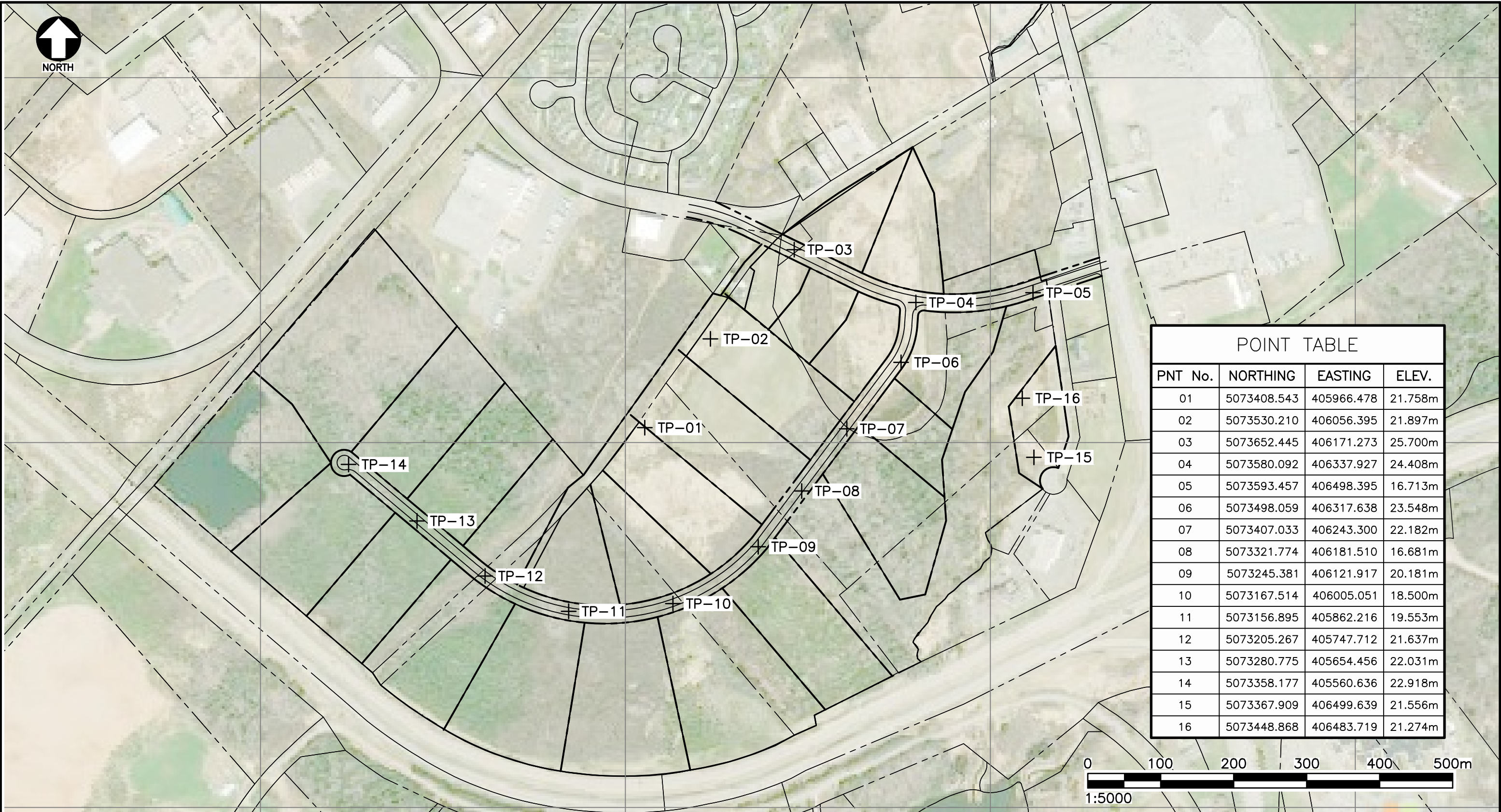
A geotechnical field investigation is a limited sampling of a site. Some variation between sampling locations should be expected. The conclusions presented in this report represent the technical judgment of CBCL, based on the data obtained from the work and on CBCL's understanding of the project. The data obtained by CBCL is specific to the time the work was performed at the specific testing and/or sampling locations and can only be extrapolated to an undefined limited area surrounding these locations. The extent of the limited area depends on the soil and groundwater conditions, as well as the history of the site reflecting natural, construction, and other activities. Due to the nature of the investigation and the limited data available, CBCL cannot and does not warrant those undiscovered environmental liabilities and/or undetected subsurface conditions may not arise.

We trust this is the information you require at this time. We are available to discuss the contents of this report at your convenience. This report was prepared by Alex Mouland, P.Eng., PMP and reviewed by Anna Shafiq, M.A.Sc., P.Eng.

APPENDIX A

Test pit Location Plan

DRAWING NAME: Y:\MONCTON\DATA\PROJECTS\2026\263404.00 - AMHERST INDUSTRIAL PARK EXPANSION\4 CAD\01 CIVIL\03 DRAWING SHEETS\263404.00 - GEOTECH TP LOCATIONS.DWG LAYOUT NAME: C01 PLOT DATE: May 13, 2026 4:21:31 PM CAD OPERATOR: MCHAVARRO



POINT TABLE			
PNT No.	NORTHING	EASTING	ELEV.
01	5073408.543	405966.478	21.758m
02	5073530.210	406056.395	21.897m
03	5073652.445	406171.273	25.700m
04	5073580.092	406337.927	24.408m
05	5073593.457	406498.395	16.713m
06	5073498.059	406317.638	23.548m
07	5073407.033	406243.300	22.182m
08	5073321.774	406181.510	16.681m
09	5073245.381	406121.917	20.181m
10	5073167.514	406005.051	18.500m
11	5073156.895	405862.216	19.553m
12	5073205.267	405747.712	21.637m
13	5073280.775	405654.456	22.031m
14	5073358.177	405560.636	22.918m
15	5073367.909	406499.639	21.556m
16	5073448.868	406483.719	21.274m

A	ISSUED FOR REVIEW
No.	Description

Date MAY 13/26	Scale AS SHOWN	Designed AM	Drawn MC	Checked AM	Approved AM	CBCL No. 263402.00	Contract -
		TEST PITS LOCATION PLAN					Drawing SK1
		TOWN OF AMHERST INDUSTRIAL PARK					

APPENDIX B

Symbols and Terms Used on Test pit Logs



SYMBOLS AND TERMS USED ON BOREHOLE AND TEST PIT RECORDS

SOIL DESCRIPTION

Terminology describing common soil genesis:

<i>Topsoil</i>	variable mixture of mineral particles and organic matter
<i>Peat</i>	decomposing vegetative matter having fibrous and/or amorphous structure
<i>Till</i>	unstratified glacial deposit which may range from clay to boulders
<i>Fill</i>	any materials below the surface identified as placed by humans (excluding buried services)

Terminology describing soil structure:

<i>Desiccated</i>	having visible signs of weathering by oxidation of clay minerals, shrinkage cracks, etc.
<i>Fissured</i>	having cracks, and hence a blocky structure
<i>Varved</i>	composed of regular alternating layers of silt and clay
<i>Stratified</i>	composed of alternating successions of different soil types, e.g. silt and sand
<i>Layer</i>	>75 mm
<i>Seam</i>	2 mm to 75 mm
<i>Parting</i>	< 2 mm
<i>Well Graded</i>	having wide range in grain sizes and substantial amounts of all intermediate particle sizes
<i>Uniformly Graded</i>	predominantly of one grain size

Terminology describing soils on the basis of grain size and plasticity is based on the ASTM D2488 – Standard Practice for Description and Identification of Soils (Visual-Manual Procedure). The classification excludes particles larger than 76 mm (3 inches). This system provides a group symbol (e.g. SM) and group name (e.g. silty sand) for identification.

Terminology describing materials outside the USCS, (e.g. particles larger than 76 mm, visible organic matter, construction debris) is based upon the proportion of these materials present:

<i>Trace, or occasional</i>	Less than 10%
<i>Some</i>	10-20%
<i>Frequent</i>	Greater than 20%

The standard terminology to describe cohesionless soils includes the compactness as determined by laboratory test or by the Standard Penetration Test 'N' – value.

Relative Density	'N' Value	Compactness %
<i>Very Loose</i>	<4	<15
<i>Loose</i>	4-10	15-35
<i>Compact</i>	10-30	35-65
<i>Dense</i>	30-50	65-85
<i>Very Dense</i>	>50	>85



The standard terminology to describe cohesive soils includes the consistency, which is based on undrained shear strength as measured by in-situ vane tests, penetrometer tests, unconfined compression tests, or occasionally by standard penetration tests.

Consistency	Undrained Shear Strength (Su)		'N' Value
	Kips/sq.ft.	KPa	
<i>Very Soft</i>	< 0.25	< 12.5	< 2
<i>Soft</i>	0.25 – 0.5	12.5 – 25	2 – 4
<i>Firm</i>	0.5 – 1.0	25 – 50	4 – 8
<i>Stiff</i>	1.0 – 2.0	50 – 100	8 – 15
<i>Very Stiff</i>	2.0 – 4.0	100 – 200	15 – 30
<i>Hard</i>	> 4.0	> 200	> 30

ROCK DESCRIPTION

Rock Quality Designation (RQD)

The classification is based on a modified core recovery percentage in which all pieces of sound core over 100 mm long are counted as recovery. The smaller pieces are considered to be due to close shearing, jointing, faulting, or weathering in the rock mass and are not counted. RQD was originally intended to be done on N-size (45 mm) core; however, it can be used on different core sizes if the bulk of the fractures caused by drilling stresses are easily distinguishable from in situ fractures.

RQD	ROCK QUALITY
90 – 100	Excellent, intact, very sound
75 – 90	Good, massive, moderately jointed or sound
50 – 75	Fair, blocky and seamy, fractured
25 – 50	Poor, shattered and very seamy or blocky, severely fractured
0 – 25	Very poor, crushed, very severely fractured

Terminology describing rock mass:

Spacing (mm)	Bedding, Laminations, Bands	Discontinuities
2000 – 6000	<i>Very Thick</i>	<i>Very Wide</i>
600 – 2000	<i>Thick</i>	<i>Wide</i>
200 – 600	<i>Medium</i>	<i>Moderate</i>
60 – 200	<i>Thin</i>	<i>Close</i>
20 – 60	<i>Very Thin</i>	<i>Very Close</i>
< 20	<i>Laminated</i>	<i>Extremely Close</i>
< 6	<i>Thinly Laminated</i>	

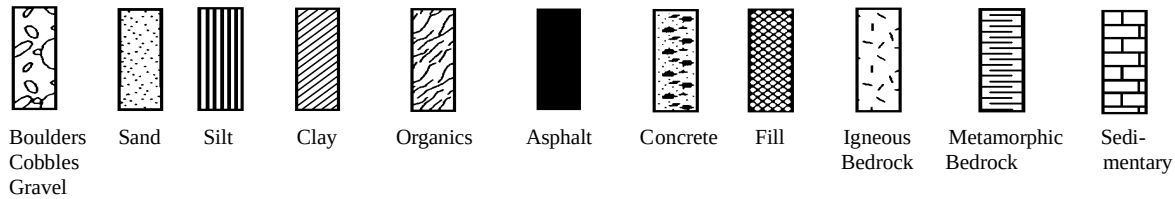
Strength Classification	Uniaxial Compressive Strength (MPa)
<i>Very Weak</i>	1 – 5
<i>Weak</i>	5 – 25
<i>Medium Strong</i>	25 – 50
<i>Strong</i>	50 – 100
<i>Very Strong</i>	100 – 250
<i>Extremely Strong</i>	> 250

Terminology describing weathering:

<i>Slight</i>	- Weathering limited to the surface of major discontinuities. Typically iron stained.
<i>Moderate</i>	- Weathering extends throughout rock mass. Rock is not friable.
<i>High</i>	- Weathering extends throughout rock mass. Rock is friable.

STRATA PLOT

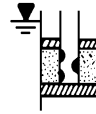
Strata plots symbolize the soil or bedrock description. They are combinations of the following basic symbols:



WATER LEVEL MEASUREMENT



Borehole or
Standpipe



Piezometer

SAMPLE TYPE AND/OR FIELD TESTS

SS	Split Spoon Sample (obtained by performing the Standard Penetration Test)	AS	Auger Sample
ST	Shelby Tube or Thin Wall Tube	BS	Bulk Sample
PS	Piston sample	WS	Wash Sample
DC	Dynamic Cone Penetration	HQ, NQ, BQ, etc.	Rock Core Samples (obtained with the use of standard size diamond drilling bits)
FSV	Field Shear Vane		

N- VALUE

Numbers in this column are the results of the SPT (Standard Penetration Test): the number of blows of a 140 pound (64kg) hammer falling 30 inches (760 mm), required to drive a 2 inch (50.8 mm) O.D. split spoon sampler one foot (305 mm) into the soil. For split spoon samples where insufficient penetration was achieved and 'N' values cannot be presented, the abbreviation SSR (Split Spoon Refusal) will appear in place of a numerical value.

OTHER TESTS

Symbols in this column indicate that the following laboratory tests have been carried out and the results are presented separately.

S	Sieve analysis	H	Hydrometer analysis
G _s	Specific gravity of soil particles	γ	Unit weight
k	Permeability	C	Consolidation
	Single packer permeability test; test interval from depth shown to bottom of borehole	CD	Consolidated drained triaxial
	Double packer permeability test; Test interval as indicated	CU	Consolidated undrained triaxial with pore pressure measurements
	Falling head permeability test using casing	UU	Unconsolidated undrained triaxial
	Falling head permeability test using well point or piezometer	DS	Direct shear
		Q _u	Unconfined compression
		I _p	Point Load Index (I _p on Borehole Records equals I _p (50); the index corrected to a reference diameter of 50 mm)
		MSV	Laboratory Miniature Shear Vane

APPENDIX C

Test pit Logs



TEST PIT TP-01

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 21.8 m

DATUM Geodetic (CGVD28)

COORDINATES 5073409 m N; 405966 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			21.8	ROOTMAT/TOPSOIL (600 mm thick)			
0.5			21.2	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL			
1.0							
1.5							
2.0			19.8				

End of test pit at 2.0 m



TEST PIT TP-02

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 21.9 m

DATUM Geodetic (CGVD28)

COORDINATES 5073530 m N; 406056 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			21.9	ROOTMAT/TOPSOIL (600 mm thick)			
0.5			21.3	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL			
1.0							
1.5							
			20.1				

End of test pit at 1.8 m



TEST PIT TP-03

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 25.7 m

DATUM Geodetic (CGVD28)

COORDINATES 5073652 m N; 406171 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			25.7	ROOTMAT/TOPSOIL (600 mm thick)			
0.5			25.1	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increase near bottom of excavation			
1.0							
1.5							
2.0			23.7				

End of test pit at 2.0 m



TEST PIT TP-04

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 24.4 m

DATUM Geodetic (CGVD28)

COORDINATES 5073580 m N; 406338 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			24.4	ROOTMAT/TOPSOIL (600 mm thick)			
0.5			23.8	Stiff, reddish brown, SILT/CLAY with sand: TILL			
1.0							
1.5							
2.0							
			22.2	End of test pit at 2.2 m			



TEST PIT TP-05

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE 2026-04-13

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 16.7 m

DATUM Geodetic (CGVD28)

COORDINATES 5073593 m N; 406498 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐
			16.7	ROOTMAT/TOPSOIL (400 mm thick)			
0.5			16.3	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - groundwater seepage at 1.0 m - gravel/cobbles content increase near bottom of excavation			
1.0							
1.5							
2.0							
2.5							
			14.1				

End of test pit at 2.6 m



TEST PIT TP-06

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 23.5 m

DATUM Geodetic (CGVD28)

COORDINATES 5073498 m N; 406318 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			23.5	ROOTMAT/TOPSOIL (500 mm thick)			
0.5			23.0	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL			
1.0							
1.5							
2.0							
2.5							
			20.8				

End of test pit at 2.7 m



TEST PIT TP-07

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 22.2 m

DATUM Geodetic (CGVD28)

COORDINATES 5073407 m N; 406243 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			22.2	ROOTMAT/TOPSOIL (600 mm thick)			
0.5			21.6	Loose, reddish brown, silty/clayey SAND			
1.0			21.2	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increase near bottom of excavation			
1.5							
2.0							
2.5			19.7				

End of test pit at 2.5 m



TEST PIT TP-08

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 16.7 m

DATUM Geodetic (CGVD28)

COORDINATES 5073322 m N; 406182 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			16.7	ROOTMAT/TOPSOIL (600 mm thick)			
0.5			16.1	Greyish brown, silty/clayey sand to sandy silt/clay: FILL			
1.0			15.7	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increases near bottom of excavation			
1.5							
2.0							
			14.4				

End of test pit at 2.3 m



TEST PIT TP-09

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 20.2 m

DATUM Geodetic (CGVD28)

COORDINATES 5073245 m N; 406122 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			20.2	ROOTMAT/TOPSOIL (500 mm thick)			
0.5			19.7				

Inferred BEDROCK. End of test pit at 0.5 m



TEST PIT TP-10

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 18.5 m

DATUM Geodetic (CGVD28)

COORDINATES 5073168 m N; 406005 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐
			18.5				
0.5				ROOTMAT/TOPSOIL (800 mm thick)			
			17.7				
1.0				Greyish brown, silty/clayey sand to sandy silt/clay: FILL			
			17.4				
1.5				Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increase near bottom of excavation			
2.0							
2.5							
			15.8				

End of test pit at 2.7 m



TEST PIT TP-11

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 19.6 m

DATUM Geodetic (CGVD28)

COORDINATES 5073157 m N; 405862 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			19.6	ROOTMAT/TOPSOIL (300 mm thick)			
0.5			19.3	Loose to compact, reddish brown, silty/clayey SAND with gravel			
1.0							
1.5							
2.0							
2.5			17.4	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL			
			17.1				

End of test pit at 2.5 m



TEST PIT TP-12

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-13

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 21.6 m

DATUM Geodetic (CGVD28)

COORDINATES 5073205 m N; 405748 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			21.6	ROOTMAT/TOPSOIL (300 mm thick)			
0.5			21.3	Loose, reddish brown, silty/clayey SAND			
1.0			20.8	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL			
1.5							
2.0							
			19.4				

End of test pit at 2.2 m



TEST PIT TP-13

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-14

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 22.0 m

DATUM Geodetic (CGVD28)

COORDINATES 5073281 m N; 405654 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			22.0	ROOTMAT/TOPSOIL (300 mm thick)			
			21.7	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increases near bottom of excavation			
0.5							
1.0							
1.5							
2.0							
2.5							
			19.2				

End of test pit at 2.8 m



TEST PIT TP-14

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-14

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 22.9 m

DATUM Geodetic (CGVD28)

COORDINATES 5073358 m N; 405561 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			22.9	ROOTMAT/TOPSOIL (300 mm thick)			
0.5			22.6	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increase near bottom of excavation			
1.0							
1.5							
2.0							
			20.6				

End of test pit at 2.3 m



TEST PIT TP-15

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-14

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 21.6 m

DATUM Geodetic (CGVD28)

COORDINATES 5073368 m N; 406500 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
			21.6	ROOTMAT/TOPSOIL (300 mm thick)			
			21.3	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL			
0.5							
1.0							
1.5							
2.0							
			19.4				

End of test pit at 2.2 m



TEST PIT TP-16

PAGE 1 OF 1

CLIENT Town of Amherst

PROJECT NUMBER 263404.00

DATE 2026-04-14

WATER TABLE DATE -

CONTRACTOR Beale and Inch

PROJECT NAME Amherst Industrial Park Expansion

PROJECT LOCATION Amherst, NS

GROUND ELEVATION 21.3 m

DATUM Geodetic (CGVD28)

COORDINATES 5073449 m N; 406484 m E

LOGGED BY JA CHECKED BY AM

DEPTH SCALE (m)	WATER LEVEL (m)	GRAPHIC LOG	ELEVATION (m)	MATERIAL DESCRIPTION	SAMPLE TYPE	SAMPLE NUMBER	PL MC LL 20 40 60 80 ☐ FINES CONTENT (%) ☐ 20 40 60 80
0.5			21.3	Reddish brown, silty/clayey sand with gravel and cobbles: FILL - black organics between 0.7 - 0.9 m			
1.0			20.4	Compact, reddish brown, silty/clayey SAND with gravel and cobbles: TILL - gravel/cobbles content increases near bottom of excavation			
1.5							
2.0							
2.5							
3.0			18.3				

Probable BEDROCK at bottom of excavation. End of test pit at 3.0 m

APPENDIX D

Guidelines for Winter Construction



GEOTECHNICAL RECOMMENDATIONS FOR WINTER CONSTRUCTION

The following are general geotechnical recommendations for earthworks for building areas in winter conditions.

General

- ▶ Earthworks conducted during freezing conditions are suspect. Special procedures and precautions must be exercised to minimize the risk of future problems.
- ▶ A site meeting should be held at project start-up to discuss the schedules of the various contractors in relation to the following geotechnical recommendations.

Excavation

- ▶ The rootmat/topsoil layer and any overlying snow will reduce the frost penetration. Conducting only the excavation work required for each day of work is recommended to minimize freezing of the soil in the foundation areas.
- ▶ Excavated material to be used as structural fill should not be stockpiled but should be placed and compacted immediately after excavation.

Fill Placement

Based on our experience, it is generally impractical to place well-graded gravel, sand, or fine-grained soils in temperatures lower than about -5 degrees Celsius. On very cold days, loose material starts to freeze within about 15 minutes. At temperatures below -5 degrees Celsius, clear gravel or clear rockfill is recommended but subject to design considerations governing the work.

The following provides recommendations for all structural fill types.

- ▶ Structural fill placement should be conducted in small areas. Depending on the temperature, this may allow for continuous placement of fill lifts during the workday without the requirement for excavation of frozen material prior to placement of the next lift.
- ▶ Material containing snow or ice should not be incorporated in the work. During snow events, fill placement should be stopped. When the earthwork restart, all snow and ice should be removed from the fill surface prior to subsequent fill placement. In order to remove all snow and/or ice after a snow event, some of the underlying fill may have to be removed and wasted.
- ▶ For intermediate fill lifts, frost protection (e.g., straw, insulated tarp, etc.) should be provided at the end of the workday, or alternatively, fill that freezes overnight should be removed in the morning. Also, any snow or ice should also be removed. Fill surfaces should be sloped to prevent ponding of water during milder weather.

- ▶ The final fill surface, the base of footing excavations and slab subgrade should be protected from freezing. If the final fill surface is exposed to freezing temperatures, heat will be required to thaw the soil. Test pits and temperature readings could be completed to determine if the soil is above freezing. Consideration should also be given to the installation of thermocouples in the fill during placement, as a means of reading temperatures at depth. The areas that were frozen should be proof-rolled.
- ▶ The moisture content of fill materials should be approximately 2% below optimum. Fill materials with moisture contents above the optimum should not be used.
- ▶ Loose edges of the structural fill lifts should be avoided to reduce frost penetration. Edges of fill lifts should be tapered and compacted.
- ▶ Regular checks of the temperature of the fill should be made. The soil temperature should be greater than +2°C to allow for compaction to the specified degree.

Footing Construction

- ▶ Footings should not be placed on frozen material.
- ▶ Where the footing elevation is within approved finer-grained materials, we recommend over-excavation by at least 6 inches and placement of nominal 1 inch stone or other clean gravel. This will reduce disturbance of the bearing surface.
- ▶ Following construction of footings, temporary frost protection must be provided to avoid freezing of the bearing surface and for protection of the concrete during curing.
- ▶ Consideration should be given to specifying that the footing depth for interior foundations be 1.2 m below slab subgrade for frost protection during construction; or alternatively, fill could be temporary bermed over interior footings to provide insulation.
- ▶ Foundations should be backfilled with a free-draining granular material and drainage provided to prevent adfreeze of foundations, particularly during construction.
- ▶ Cast-in-place concrete should be protected during colder weather conditions as per CSA A23.1-2014.

Geotechnical Inspection and Testing

The information herein should be reviewed by geotechnical personnel and customized to the specific geotechnical aspects and design considerations of a site. Full-time inspection and testing by experience geotechnical personnel is particularly important during earthworks in winter conditions and is strongly recommended.



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